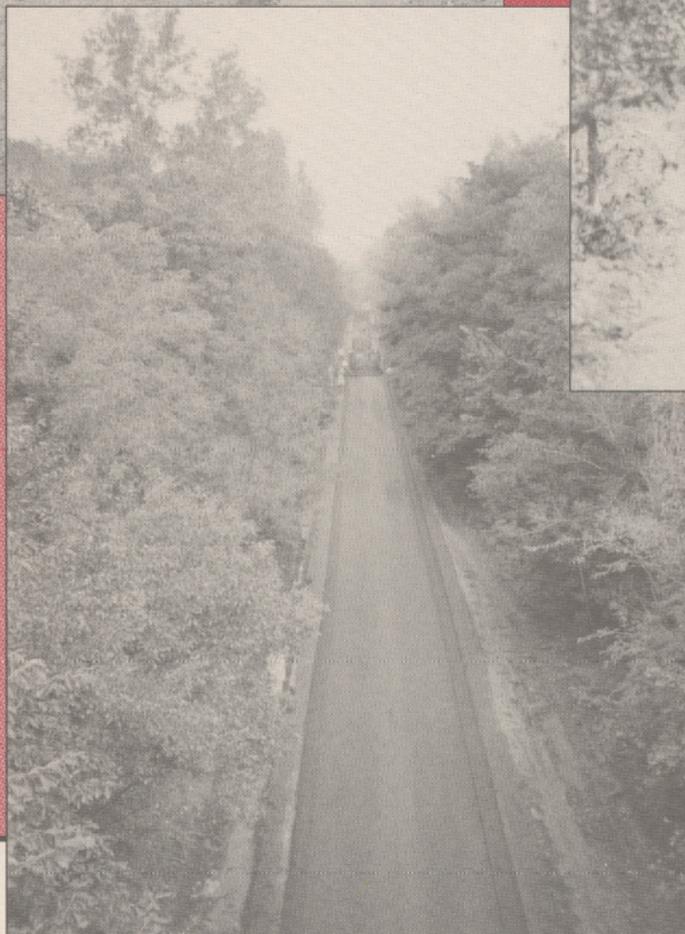


91-06

Waste Tire and Shingle Scrap/Bituminous Paving Test Sections



on the
Willard Munger
Recreational Trail
Gateway Segment

Interim Report



A Cooperative Study Performed by
Minnesota Pollution Control Agency
Minnesota Department Of Natural Resources
Minnesota Department of Transportation



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**Waste Tire and
Shingle Scrap/Bituminous
Paving Test Sections
on the
Willard Munger
Recreational Trail
Gateway Segment**

Interim Report

Construction Report by
Curtis M. Turgeon
Research Project Engineer
Minnesota Department of Transportation

A Cooperative Study Performed by
Minnesota Pollution Control Agency
Minnesota Department Of Natural Resources
Minnesota Department of Transportation

ABSTRACT

The need to reduce our states dependence on land fills resulted in a unique cooperative venture by three state agencies. A partnership was forged between the Minnesota Pollution Control Agency (MPCA), the Minnesota Department of Natural Resources (DNR) and the Minnesota Department of Transportation (Mn/DOT) to investigate the use of recycled tire rubber and processed asphalt shingle scrap. The result is a two mile section of the Willard Munger Recreational Trail in St. Paul constructed with asphalt paving mixtures which contain varying percentages of recycled tire rubber and shingle scrap.

Special bituminous mix designs were formulated using 3% rubber, 6% rubber, 3% rubber with 6% shingles and 9% shingles. The mixtures containing rubber did not exhibit acceptable mix characteristic values under present Mn/DOT bituminous specifications. The shingle-only mix met specifications and yielded an economic advantage of decreasing the asphalt cement demand of the mix.

Conventional mixing and paving equipment was utilized for construction. This application appears to be a viable alternative to landfilling these materials. However, costs for the mixtures containing rubber increased from 35% to 50% over the cost of the conventional mixture. Since the use of shingle scrap was negotiated by the private companies involved, no comparable cost data is available.

ACKNOWLEDGEMENTS

The author would like to acknowledge the contributions of the following individuals and their organizations. An immense amount of cooperation and coordination was required to implement this concept.

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Tim Peterson, MDNR
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Roger Olson Mn/DOT
Doug Schwartz, Mn/DOT

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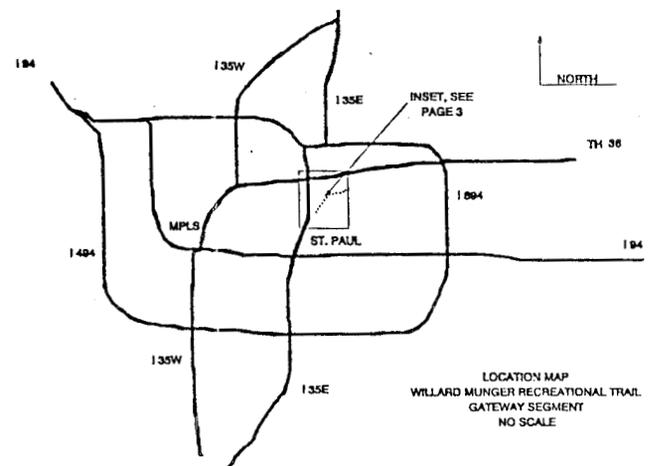
Background.

The need to reduce our states dependence on land fills resulted in a unique cooperative venture by three state agencies. A partnership was forged between the Minnesota Pollution Control Agency (MPCA), the Minnesota Department of Natural Resources (DNR) and the Minnesota Department of Transportation (Mn/DOT) to investigate the use of recycled tire rubber and processed asphalt shingle scrap. The result is a two mile section of the Willard Munger Recreational Trail in St. Paul constructed with asphalt paving mixtures which contain varying percentages of recycled tire rubber and shingle scrap.

The location of the Gateway Segment, which contains the test sections, is shown in Figure 1. The test sections contain the following proportions of waste materials:

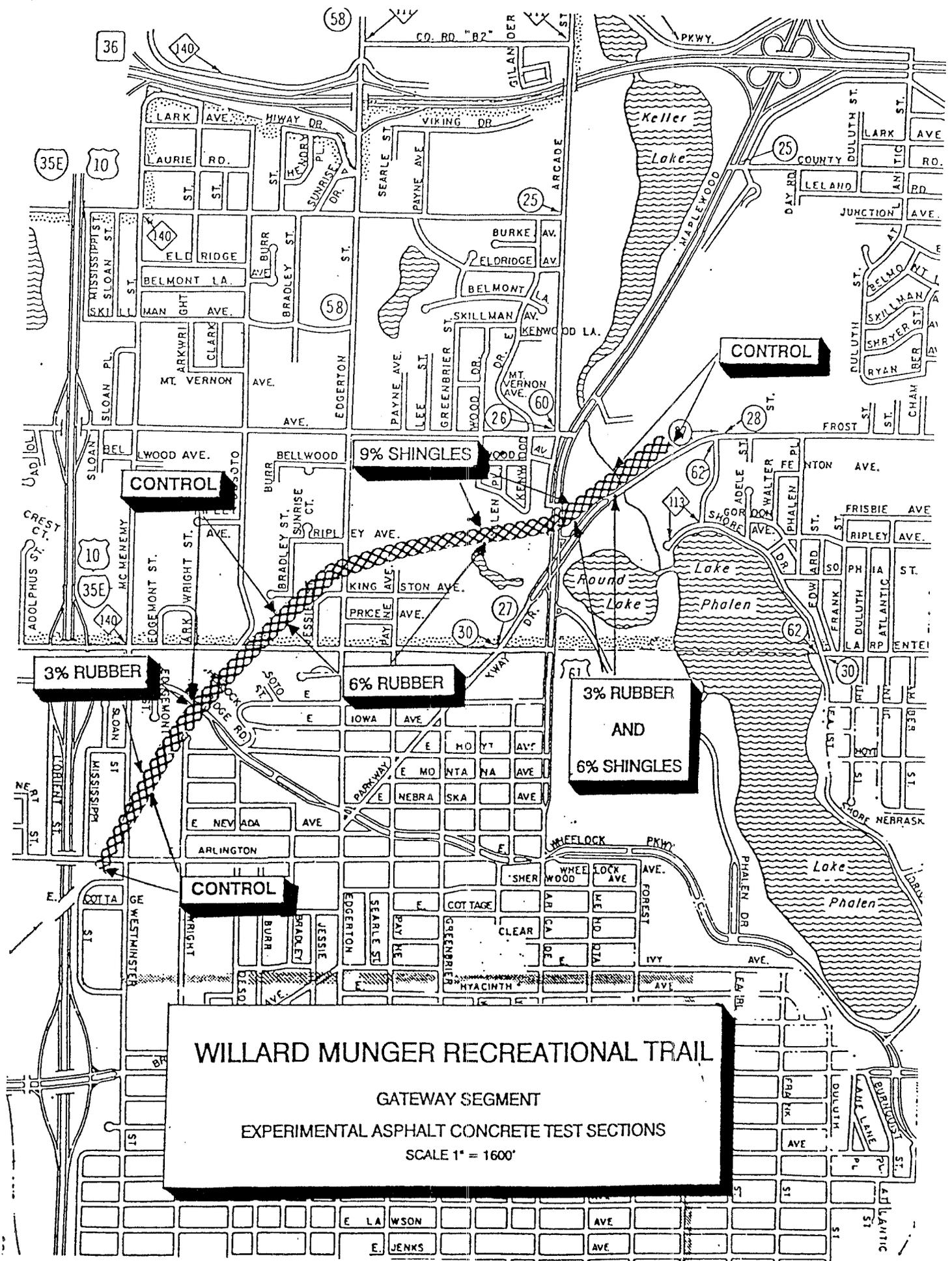
- 3% Rubber
- 6% Rubber
- 3% Rubber and 6% Shingle Scrap
- 9% Shingle Scrap

(Percent by weight of the mineral aggregates.) Asphalt mixture utilizing only standard aggregates was also used as a control mix for comparison purposes. Locations of the test and control sections are shown on the following page.



Since 1985, when the landfilling of waste tires became illegal, there have been ongoing efforts to find viable markets for waste tire products. Presently, most of Minnesota's waste tires are shredded and consumed as industrial boiler fuel. The use of ground tire rubber in asphalt paving mixes is not new. Past experiences by Mn/DOT and other agency's had shown them to be constructable. However, highway experiences demonstrated less room for error when placing rubber mixtures and, when errors occurred, catastrophic failures ensued. Rubber mixtures also require a higher percentage of asphalt cement. Since asphalt cement is by far the most expensive ingredient in an asphalt paving mixture, higher asphalt demand equates to a significant rise in construction cost.

The concern over potential failures led to the concept of testing the mixtures on recreational trails instead of on highway construction. Using the trail allowed a more bold approach in formulating the mixture designs since reconstruction of failed test sections would not pose the traffic control problems and other costs associated with highway testing. However, the trail pavement is subjected to exactly the same mixing, paving and, most importantly, environmental factors common to all asphalt paving.



WILLARD MUNGER RECREATIONAL TRAIL
 GATEWAY SEGMENT
 EXPERIMENTAL ASPHALT CONCRETE TEST SECTIONS
 SCALE 1" = 1600'

The use of shingle scrap was identified as a means to reduce the mixtures asphalt demand and make the use of rubber more economical. Organic and fiberglass shingles, the two types presently produced in the United States, contain approximately 30% and 19% asphalt cement, respectively. Ends of runs, samples, off color shingles and tabs create approximately 5% waste during the manufacturing process. This waste is presently landfilled. It is important to note that the shingle scrap was obtained directly from a shingle manufacturing plant. It did not include waste removed during reroof construction. While utilization of reroof waste has merit, problems with uniformity, removal of nails and material separation preclude its use at this time.

Laboratory Bituminous Mix Design.

Previous studies and background testing had shown that mixes containing ground rubber display lower Marshall stability (a standard measure of resistance to deformation or strength) and higher air void content than conventional mixtures. Upon awarding the contract to the low bidder, Bituminous Roadways, samples of standard aggregates were obtained from their stockpiles. It is essential that the actual aggregates to be used on any bituminous project are used in the trial mix testing.

The DNR specifications called for the use of standard Mn/DOT mix type 2341. Trial mix lab work performed in the Mn/DOT Central Lab yielded the following mix designs.

% Agg	% Rubber	% Shingles	% Asphalt	Stab. lbs.	% Air Voids
100	0	0	5.0	1560	4.2
97	3	0	6.5	192	5.7
93	6	0	7.7	50	9.1
91	3	6	5.9	408	5.3
91	0	9	3.0	2464	3.3

% Aggregate + %Rubber + % Shingles = 100% weight of dry mineral materials.

% Asphalt = % by weight of total mixture (asphalt cement and mineral materials)

Some stability and air void data was interpolated from actual test results. Quantities shown are for comparison purposes only.

Mix design sheets are in Appendix A of this report.

The Mn/DOT bituminous specification type 2341 for 1990 required a minimum Marshall stability of 1000 lbs. and a maximum of 3000 lbs with a targeted air void content of 4.0.

All mixes contained the same combination of natural aggregates. The control mix met type 2341 gradation requirements but was formulated to be on the coarser side of the tolerances. Experience with other aggregates had shown that when rubber is added, relatively coarser aggregates yield higher stabilities. It is theorized that larger stones maintain better aggregate interlock as the smaller less dense rubber particles fill the gaps. Even with this slight beneficial influence, rubber-only mixtures exhibited extremely low stability values.

The stability values for the rubber and shingle mix were somewhat better, but still failed to meet specification. A notable decrease in asphalt demand, 0.6 percent, was also demonstrated. The shingle-only mix met specification and demonstrated a significantly higher Marshall stability than the control mixture. The angular granules and relatively hard asphalt cement contributed by the shingle scrap are potential sources of this increase in stability. The decrease of 2.0 percent in asphalt demand displayed has important potential economic benefits.

Pre-Construction.

A great deal of planning was required to bring together the information and materials to make this project work. The DNR prepared the plans and specifications, with assistance from Mn/DOT. The MPCA contracted with the Trash Depot Inc of Moorhead, Minnesota to produce and deliver the ground rubber. The use of shingle scrap was coordinated by the J.L. Shiely Company. This including locating a shingle scrap source; Certainteed Inc of Shakopee, Mn; selecting a processor/grinder, the Omann Brothers of St. Michael, Mn; and working with the paving contractor, Bituminous Roadways. Performing the mix design testing was provided by Mn/DOT.

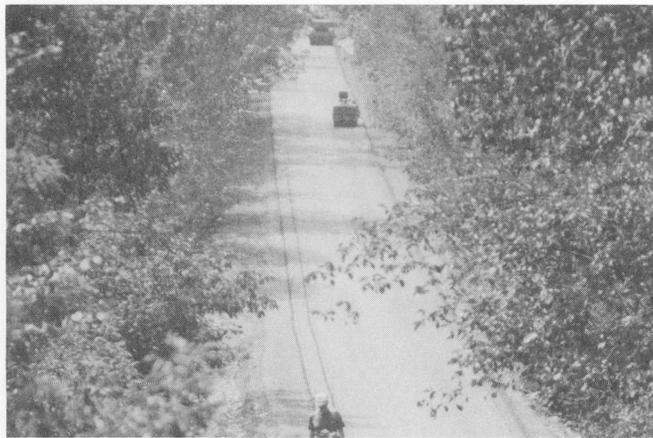
Construction.

This section of trail was placed on abandoned Soo Line Rail Road right of way. The in-place track bed was reshaped as needed and a 4" thick crushed concrete base was placed and compacted.

A batch-type plant was used to prepare the mixture. All waste materials were introduced through the plant's recycled asphalt pavement inlet. No recycled asphalt pavement was utilized on this project. Higher mixing temperatures (30-40 degrees F higher than the normal 290 F) and slightly longer mixing times were utilized to foster better rubber/aggregate coating. Since only one inlet was available for waste product introduction, premixing of the rubber and shingles for the rubber-shingle mix was required. This was accomplished with a front-end-loader and truck scale. The

contractors versatility and previous experience with rubber athletic tracks led to relatively smooth production with few unexpected plant problems.

The weather during construction was hot and humid with clear skies and highs in the 90's F. The 12 foot wide, 2.5 inch thick mat was place in one paver pass. A steel wheeled roller provided compaction with a second smaller steel wheeled roller creating the finished surface. The use of pneumatic tired roller was no recommended due to potential rubber pickup problems. A few 2-3 inch diameter clumps of shredded shingles appeared during paving. While the clumps posed only a minor problem, a process to break up or remove them should be adopted.



Performance.

The surface texture immediately after construction was somewhat open and porous. Much of this was due to the coarse natural aggregate gradation used in hopes of gaining stability. It appears that what little was gained in stability was sacrificed in terms of surface texture.

Shortly after construction some loss of rubber particles from the surface occurred. As yet, this phenomenon has been minimal and has not significantly effected the surface texture.

While the shingle-only mixture also exhibited a relatively open surface texture due to the coarse natural aggregates, it is performing satisfactorily to date.

Laboratory Analysis.

Core sample were taken from each test section. The following parameters were evaluated in the Mn/DOT lab.

Mix Type	Density (Bulk) lbs/cu.ft	Split Tensile Strength psi. avg. / range	Inplace Air Voids (Rice)	%AC	AC PEN
Control	141.7	70 / 64-76	9.0	5.3	52
3% Rubber	128.8	42 / 28-50	12.6	6.3	76
6% Rubber	122.7	30 / 29-31	13.0	7.8	111
3% Rubber 6% Shingles	129.6	40 / 34-48	12.6	7.3	55
9% Shingles	130.5	37 / 31-48	16.1	5.4	34

%AC = Percent of extracted asphalt cement by weight mix.
AC PEN = Penetration of recovered asphalt at 77 F.

(Penetration is a relative measure of the stiffness of the asphalt cement. The term "recovered" refers to the process of washing the asphalt from the aggregate with a solvent. The asphalt and solvent are then separated and the asphalt is tested.)

One should exercise discretion when comparing the above data since the tests are formulated for standard asphalt-aggregate mixtures.

The control mix exhibited relatively standard results. The tensile strength is somewhat low but this may be due to the coarseness of the aggregates. The grade of asphalt cement used was 120-150, which means its penetration prior to mixing fell between 120 and 150. The heat applied during mixing causes the asphalt to stiffen, hence the recovered (after mixing and placement) penetration of 52.

The rubber-only mixtures have in general lower densities, lower tensile strengths, higher air voids and have asphalt contents close to the prescribed contents in the mix design formulation. The penetration values seem to indicate some resistance to the normal asphalt hardening due to heating and mixing. The rubber may be soaking up and "hiding" the asphalt from the heat. However the rubber may be reacting with the solvent in the extraction/recovery process and tainting the results. Further studying is needed to clarify this point and what effect it has on mix properties.

The rubber-shingle mix yielded results similar to the rubber-only mixes with the notable exception of the recovered penetration. The asphalt cement contributed by the shingles is relatively hard. (low penetration) Therefore the combination of the standard asphalt and

shingle asphalt creates a harder binder.

The effect of the stiff shingle asphalt is more apparent in the shingle-only mix. The penetration of the shingle-only mix is lower than the control mix. Stiffer binders can be more susceptible to low temperature cracking. Cracking of the shingle-only mix should be monitored closely. Inplace air voids are higher than expected as well. The stiffness of the asphalt cement may also inhibit compaction.

Cost.

Precise costs are difficult to establish due to the wide variety of organizations contributing to the project. The MPCA contract, which included purchase and delivery of the ground rubber, yielded the following totals:

Tons of rubber delivered:	38.0
Price per ton:	\$ 125.00
Total Contract:	\$4,750.00

The unit prices in the DNR contract for mixing and placing the control and rubber mixes were as follows:

Control mix	\$ 3.60 /sq yd
3% Rubber	\$ 4.40 /sq yd
6% Rubber	\$ 4.50 /sq yd

When the rubber purchase and delivery is factored into the mixing and placing costs the total costs become:

Control mix	\$ 3.60 /sq yd
35 Rubber	\$ 4.85 /sq yd
6% Rubber	\$ 5.41 /sq yd

One can see that the total cost for the rubber mixtures is 35% to 50% higher than the control mix.

The concept of using shingle scrap developed after the above contracts were awarded. All cost for transport, processing, mixing and placing were negotiated between the private companies involved. All materials and processing were provided to the State at no cost. It is possible that the savings from the decrease in asphalt demand would offset any handling or processing costs. Depending upon the price of asphalt cement, shingle scrap use may actually decrease the bituminous mixes total cost.

Conclusions.

1. This project has shown bituminous trail construction with two waste products, ground tire rubber and shredded shingle waste, to be a viable alternative to landfill disposal.
2. Laboratory characteristics of bituminous mixtures containing

ground tire rubber did not favorably compare to the control mix or to the Mn/DOT specifications. Rubber mixtures exhibited high air void contents, low Marshall stabilities and high asphalt cement demand.

3. Improved Marshall stability can be achieved by using a coarser natural aggregate gradation. Unfortunately this causes the pavement surface to be open or porous in appearance.
4. Ground shingle scrap effectively reduced asphalt demand and increased Marshall stability.
5. Analysis of core test section samples removed after construction displayed low density, low tensile strength and high air voids when compared to the control mix. Mixes containing rubber had higher recovered asphalt penetration and mixes containing shingles had lower recovered asphalt penetrations when compared to the control mix.
6. The total cost for using the 3% and 6% rubber-only mixes was 35% and 50% higher, respectively, than the control mix. No expense for the use of shingles was born by the State, therefore these costs are not reported.

Recommendations.

1. The test sections should continue to be monitored for surface abrasion, cracking and general performance.
2. The natural aggregate selection and mix design process for rubber bituminous mixes for trails should focus equally on standard laboratory data (Marshall stability, air void content, asphalt demand) and potential surface texture/porosity.
3. A cost comparison/analysis should be undertaken to determine if this is a cost effective/competitive means of waste tire disposal.
4. Further testing on the use of shingle scrap in bituminous mixes is warranted.

APPENDIX A
BITUMINOUS TRIAL MIX RESULTS
EXTRACTION AND GRADATION RESULTS

BITUMINOUS PAVING RECOMMENDATION # 0-



Minnesota Department of Transportation
Materials and Research Laboratory
1400 Gervais Avenue
Maplewood, MN 55109

Date: 8/23/90
Phone: 612-779-5614
FAX: 612-779-5580

To: _____, Engineer, _____

The mix design for Spec. 2340-Special, Mixture Type 4/5 is hereby approved for this project as follows:

S.P. 9
S.A.P. 1 2 11 12 20
J O B - M I X FORMULA

Table with columns: Begin with test number, Formula, Sieve Size, Working Range. Includes rows for Base, Binder, Leveling, Wearing, and Shoulders.

Trial Mix No. 0-90191 indicates a Marshall Density of 137.3 PCF at 50 blows per side.

Table with columns: Proportions, Source of Material, BA or BR#. Rows include Solberg-Nininger, Bituminous Roadways, Shiely-Larson, and Trash Depot.

Remarks: Obtain 60# Sample & send to Maplewood Lab c/o R. Walkosz/C. Turgeon for further testing. Mix will be tender until cooled below 140°F.

Approved by:

cc: Dist. Mat'ls Engr. (Dist.) (2)
Bituminous Office (3)
Ft. Snelling (3)
Contractor

Assistant Bituminous Engineer

Project No.	9 PR 1008-0303	AB No.		Source		Proportion	
Location	Ramsey	T.H. No.	90392	Solberg		9	
Additive	%		393	Rit Rdways		59	
AC Source	Ashland		394	Shiely/Hanson		29	
AC Grade	120/150		397	Trash Rubber		3	
AC Sp.G.	1.017						

Tests:	B.I.	Mr	Ts	Extraction	Gradation	Other	
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Marshall ID	MIX A 620.9 date: 8-11				MIX B date:			
	wt. AC	% AC	# Blows		wt. AC	% AC	# Blows	
Height	235/64	235/64	2 13/32		233/64	233/64	2 1/2	
Dry Wght.	1098.4	1132.3	1129.3		1123.8	1125.2	1119.9	
SSD Wght.	1107.2	1137.9	1137.1		1127.8	1128.3	1123.4	
Imm. Wght.	598.6	622.9	619.2		617.5	619.2	614.6	
Volume	508.6	515.0	517.9		510.3	509.1	508.8	
Bulk Sp.G.	2.160	2199	2181	2180	2202	2210	2201	2204
Density	135.8							137.3
Flow	20	25	20	22	15	16	16	16
Stability Dial	1100	2100	1600		2100	2100	1600	
Stability	106.7	203.7	153.6	154.7	207.9	207.9	160.0	191.9

Container ID		C		D
Cont. + Sa Dry	K			K
Cont. Dry	L	742.1		L
Sa. Dry	M	2016.1		M
Cont. + Sa Imm.	N	1328.9		N
Cont. Imm.	P	162.1		P
Sa Imm.	Q	1166.8		Q
Sa Volume	R	849.3		R
Max. Sp.G.	S	2.374	2374	S
Rice Voids	T		8.2	T

	Abrasions	2.586	Abras.	2.573	Abrasions
Pan Agg Dry	Target	AC ratio	b/c		210.6
Pan Dry	(set G)	Agg ratio	100-b/n		
Agg Dry	Dry Actual		p+q		1062.1
Pan AC Dry	SSD	T.M. Sp.G.	100/r		1074.1
Pan AC Imm.	Imm	V.F. MIX	(E/s) x 100		511.5
Pan AC Volume	Orig	Vol. AC	E x P		1140.1
Pan AC Agg Dry	Final	V.M.A.	u+y		893.1
Pan AC Agg Imm.	Loss	V.F. Agg.	(4%) x 100		247.0
Pan AC Agg Vcl.	j-k	BI			21.4
Agg Volume	% Loss	Voids	100-t		
Agg Sp.G.	1/m				

Signed *R.J. Walker* date: 8/23

BITUMINOUS PAVING RECOMMENDATION # 0-



Minnesota Department of Transportation
 Materials and Research Laboratory
 1400 Gervais Avenue
 Maplewood, MN 55109

Date: 8/23/90
 Phone: 612-779-5614
 FAX: 612-779-5580

To: _____, Engineer, _____

The mix design for Spec. 2340-Special, Mixture Type 415 is hereby approved for this project as follows:

S.P. 9 _____ J O B - M I X FORMULA
 S.A.P. _____
 1 2 _____ 11 12 _____ 20

Begin with test number:

Formula

Sieve Size

Working Range

For	(Card)	Formula	P	Sieve Size	33	38
Base	41 B B 2 0 0 21 _____ 27	100	er	1 1/2"	1 0 0	1 0 0
Binder	41 B I 2 0 0 21 _____ 27	100	c	3/4"	1 0 0	1 0 0
Leveling	41 L V 2 0 0 21 _____ 27	100	e	5/8"	1 0 0	1 0 0
Wearing	41 W E 2 0 0 21 _____ 27	96	n	1/2"	1 0 0	1 0 0
Shoulders	_____ S H _____ 21 _____ 27	82	t	3/8"	9 0	1 0 0
		53	p	#4	4 7	5 9
		43	a	#10	3 9	4 7
		23	s	#40	1 9	2 7
		4.1	s	#200	2 .	6 .
		7.7	Target (New)	% A.C.	7.4	8.0

Use 120 / 150 penetration grade asphalt.

Trial Mix No. 0-90192 indicates a Marshall Density of 127.0 PCF at 50 blows per side.

Proportions	Source of Material	BA or BR#
8 %	Solberg-Ninininger (3/4" Rock)	0-90392
58 %	Bituminous Roadways (3/4" Minus)	0-90393
28 %	Shiely-Larson (1/2" Rock)	0-90394
6 %	Trash Depot (Rubber)	0-90397
%		0-

Remarks: Obtain 60# Sample, send to Maplewood Lab c/o C. Turgeon/R. Walkosz.
Mix will be tender until cooled below 140° F.

Approved by:

cc:
 Dist. Mat'ls Engr. (Dist.) (2)
 Bituminous Office (3)
 Ft. Snelling (3)
 Contractor

Assistant Bituminous Engineer

Project No.	9 PR 1008 - 0303	AB No.		Source		Proportion
Location	Ramsey	T.H.No.	90392	Solberg		8
Additive	%		393	Bit Rdngs		58
AC Source	Ashland		394	Shiely-Larson		28
AC Grade	120/150		397	Trash Rubber		6
AC Sp.G.	1.017					

Tests		B.I.	Mr	Ts	Extraction	Gradation	Other
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	MIX A 695.2 date: 8/12				MIX B 8425 date:						
	wt. AC % AC	b	# Blows		wt. AC % AC	b	# Blows				
Marshall ID		1	2	3		1	2	3			
Height	a	2 39/64	2 39/64	2 20/32	a	2 18/32	2 17/32	2 17/32			
Dry Wght.	A	1090.5	1094.0	1090.9	A	1074.5	1070.1	1075.4			
SSD Wght.	B	1098.5	1001.7	1100.0	B	1081.2	1075.3	1082.5			
Imm. Wght.	C	564.6	567.6	557.1	C	553.5	550.2	555.9			
Volume	B-C	D	533.9	534.1	542.9	D	527.7	525.1	526.6		
Bulk Sp.G.	A/D	E	2043	2048	2009	2046	E	2036	2038	2042	2039
Density	E x 62.3	F				127.5	F				127.0
Flow			15	10	10			11	11	11	
Stability Dial	chart x ht. cor.		100	50	50			50	50	50	
Stability						67					50
Container ID			F					F			
Cont. Sa Dry	K							K			
Cont. Dry	L		780.1					L			776.5
Sa. Dry	K-L	M	2020.8					M			2020.6
Cont. Sa Imm.	N		1330.4					N			1314.0
Cont. Imm.	P		196.4					P			193.9
Sa Imm.	N-P	Q	1134.0					Q			1120.1
Sa Volume	M-Q	R	886.8					R			900.5
Max. Sp.G.	M/R	S	2.279			2.279		S			2.244
Rice Voids	100 x (S-E)/S	T				10.2		T			9.1
Pan Agg. Dry	target	d	2.494			Abras		2.498			Abras
Pan Agg. Dry	set 6	e				195.6					194.8
Agg. Dry	Dry Actual	f				1173.6					1168.8
Pail AC Dry	SSD	g				1192.4					1189.2
Pail AC Imm.	Imm	h				1203.0					1205.9
Pail AC Volume	Orig h	i				496.5					531.4
Pail AC Agg. Dry	Final	j				1336.3					1293.0
Pail AC Agg. Imm.	Loss	k				950.0					1090.0
Pail AC Agg. Vol.	j-k	l				386.3					203.0
Agg. Volume	% Loss	m				28.9					15.7
Agg. Sp.G.	1/m	n									

Signed *R. J. Walker* date: 8/23

Specification No. **415** Previous Mix No.

Project No. 99R 1008 0303	AB No.	Source	Proportion
Location Ramsey	90392	Solberg	7
Additive %	393	Bit Rubbers	57
AC Source Ashland	394	Shiely-Larson	27
AC Grade 120/150	397	Trash Rubber	3
AC Sp.G. 1.017	415	Oman Shingles	6

Tests: | B.I. | Mr | Ts | Extraction | Gradation | Other |

	MIX A date: 8/13					MIX B date: 8/21				
	wt. AC	% AC	# Blows			wt. AC	% AC	# Blows		
Marshall ID	1	2	3			1	2	3		
Height	2 19/32	2 18/32	2 18/32			2 19/32	2 18/32	2 19/32		
Dry Wght.	A 1108.9	1109.8	1114.7			A 1031.6	1031.6	1027.2		
SSD Wght.	B 1110.1	1110.8	1117.4			B 1032.0	1032.6	1027.9		
Imm. Wght.	C 593.0	594.2	597.7			C 568.8	566.3	564.7		
Volume	B-C D 515.1	516.6	519.7			D 463.2	466.3	463.2		
Bulk Sp.G.	A/D E 2.153	2.148	2.145	2.149		E 2.227	2.212	2.217	2.219	
Density	E x 62.3 F			133.9		F			138.2	

Flow		12	12	11		20	17	17	
Stability Dial		320	320	380		540	430	430	
Stability	chart x ht. cor.	304	307.2	365	325	615	490	490	532

	A					A				
Container ID	K					K				
Cont. + Sa Dry	L 943.1					L 943.1				
Cont. Dry	M 2028.0					M 2023.9				
Sa. Dry	N 108.3					N 1485.0				
Cont. + Sa Imm.	P 353.2					P 353.2				
Cont. Imm.	Q 1155.1					Q 1131.8				
Sa Imm.	R 872.9					R 892.1				
Sa Volume	S 2.323	2.311		2.323		S 2.269	2.259		2.269	
Max. Sp.G.	T 2.0			7.4		T 2.7			2.2	
Rice Voids	EFF SpG 2511			Abras 205.4		2481	Avg 2.60		Abras 212.0	

Pan Agg Dry	Target	d	AC ratio	b/c	205.4	P			212.0
Pan Dry	(set 6)	e	Agg ratio	(100-b)/n		q			1272.0
Agg Dry	Dry Act.	f		p+q		r			
Pail AC Dry	SSD	g	TMSp.G.	100/r		s			
Pail AC Imm.	Imm	h	VE MIX	(E/s) x 100		t			
Pail AC Volume	Orig	i	Vol. AC	E x P		u			
Pail AC Agg Dry	Final	j	V.M.A.	u + y		v			
Pail AC Agg Imm.	Loss	k	VE Agg.	(y/s) x 100		w			
Pail AC Agg Vol.		i-k	BI			y			
Agg. Volume	Loss	m	voids	100-t					
Agg. Sp. G.	1/m	r							

Signed *[Signature]* date: 8/23

BITUMINOUS PAVING RECOMMENDATION #0-



**Minnesota Department of Transportation
Materials and Research Laboratory
1400 Gervais Avenue
Maplewood, MN 55109**

Date: 8/23/90
Phone: 612-779-5614
FAX: 612-779-5580

To: _____, Engineer, _____

The mix design for Spec. 2340-Special, Mixture Type 415 is hereby approved for this project as follows:

S.P. 9 _____ J O B - M I X FORMULA
S.A.P. _____ 11 12 20

Begin with test number: Formula Sieve Size Working Range

For Base	(Card 1) 41 B B 4 0 0 21 _____ 27	100	P e r c e n t	1 1/2"	33	1 0 0	1 0 0	38
		100		3/4"	39	1 0 0	1 0 0	44
For Binder	(Card 2) 41 B I 4 0 0 21 _____ 27	100		5/8"	45	1 0 0	1 0 0	50
		96		1/2"	51	9 0	1 0 0	56
		84		3/8"	57	7 8	9 0	62
For Leveling	(Card 3) 41 L V 4 0 0 21 _____ 27	58		#4	63	5 2	6 4	68
		44	#10	69	4 0	4 8	74	
For Wearing	(Card 4) 41 W E 4 0 0 21 _____ 27	21	#40	75	1 7	2 5	80	
		3.6	#200	81	2 .	6 .	86	
For Shoulders	(Card 5) _____ S H _____ 21 _____ 27	6.8	% A.C.	6.5	7.1	Min.	Max.	
		Target (New)						

Use 120/150 penetration grade asphalt.

Trial Mix No. 0-90194 indicates a Marshall Density of 126.2 PCF at 50 blows per side.

Proportions	Source of Material	BA or BR#
4 %	Solberg - Nininger (3/4" Rock)	0-90392
54 %	Bituminous Roadways (3/4" Minus)	0-90393
24 %	Shiely-Larson (1/2" Rock)	0-90394
6 %	Trash Depot (Rubber)	0-90397
12 %	Oman (Shingles)	0-90415

Remarks: Obtain 60th Sample for Maplewood Lab % C. Turgeon/R. U. /kasz.
Mix will be Tended until cool

Approved by:

cc:
Dist. Mat'ls Engr. (Dist.) (2)
Bituminous Office (3)
Ft. Snelling (3)
Contractor

Assistant Bituminous Engineer

Specification No.

415

Previous Mix No.

Project No.	9PR 1008 0303	AB No.	90464	Source	Solberg - Maininger 3/4 Rock	Proportion	7
Location	Ransay (Bike Path) T.H. No.		465		Bit Rdways 3/4 Minus		57
Additive	%		463		Shirley Larson 1/2 Rock		27
AC Source	Ashtland		397		Trash Depot Rubber		9
AC Grade	120/150		415		Oran Shingles		
AC Sp.G.	1.017						

Tests: | B.I. | Mr | Ts | Extraction | Gradation | Other |

		MIX C date: <u>3/6/0</u>				MIX date: <u> </u>		
	309.3 wt AC			# Blows				# Blows
	3.0 % AC	b	3.53	50		b		
Marshall ID		1	2	3				
Height	a	2 29/64	2 29/64	2 15/32		a		
Dry Wght.	A	1176.1	1175.4	1174.4		A		
SSD Wght.	B	1177.2	1176.3	1176.3		B		
Imm. Wght.	C	675.3	675.7	674.2		C		
Volume	B-C	D	501.9	500.6	502.1	D		
Bulk Sp.G.	A/D	E	2.343	2.348	2.339	E	2.343	
Density	E x 62.3	F				F	146.0	

Flow		15	12.5	17				
Stability Dial		2510	2370	2320				
Stability	chart x ht. cor.	2585	2441	2366			2464	

Container ID		A						
Cont. Sa Dry	K					K		
Cont. Dry	L	943.1				L		
Sa. Dry	K-L	M	2015.2			M		
Cont. Sa Imm.	N		1537.0			N		
Cont. Imm.	P		353.2			P		
Sa. Imm.	N-P	Q	1183.8			Q		
Sa Volume	M-Q	R	831.4			R		
Max. Sp.G.	M/R	S	2.424			S	2.424	
Rice Voids	100 x (S-E)/S	T				T	3.3	
			2.553					

Pan Agg. Dry	Target	d	AC ratio	b/c	223.9	p		
Pan Dry	set 6	e	Agg ratio	(100-b)/n	1343.4	q		
Agg Dry	Dry Act.	f		p+q		r		
Pail AC Dry	SSD	g	T.M. Sp.G.	100/r		s		
Pail AC Imm.	Imm	h	VE MIX	(E/s) x 100		t		
Pail AC Volume	Core g	i	Vol. AC	E x P		u		
Pail AC Agg Dry	Final	j	V.M.A.	u+y		v		
Pail AC Agg Imm.	loss	k	V.F. Agg.	(y/100) x 100		w		
Pail AC Agg Vol.	j-k	l	BI			y		
Agg. Volume	loss	m	Voids	100-t				
Agg. Sp.G.	1/m	n						

Signed RJ Walker date: 8/23

STATE OF MINNESOTA
DEPARTMENT OF TRANSPORTATION

TEST REPORT ON SAMPLE OF BITUMINOUS CORE

LABORATORY----	ST. PAUL	REPORT DATE----	FEB 01, 1991
TESTS COMPLETED	01/26/91	PROJECT NUMBER-	9PR6002
SUBMITTED BY--	C TURGEON	TYPE OF CONST.-	2341
INSPECTOR-----		COURSE-----	
DATE SAMPLED--		STATION NO.----	
DATE RECEIVED-	01/07/91	FIELD ID-----	2,3,4

TESTS REQUIRED: EXTRACTION GRADATION SPECIAL TESTS
COMMENTS: BIKEPATH 2,3,4

TEST RESULTS

SIEVE ANALYSIS(SQUARE OPENINGS)

% PASSING

PASS 1 IN. SIEVE--		BITUMEN(%)-----	6.2
PASS 3/4 IN. SIEVE		MOISTURE(%)-----	
PASS 5/8 IN. SIEVE	100.0	VOLATILE(%)-----	
PASS 1/2 IN. SIEVE	97.0	TESTS ON RECOVERED ASPHALT	
PASS 3/8 IN. SIEVE	81.0	PENETRATION 77 F.--	76
PASS #4 SIEVE-----	50.0	DUCTILITY 77 F.(CM)	
PASS #10 SIEVE-----	40.0	SOFTENING POINT, F.	
PASS #20 SIEVE-----	32.0	KVISC 275 F. CS----	
PASS #40 SIEVE-----	24.0	AVISC 140 F. POISES	1153
PASS #80 SIEVE-----	9.0	RICE VOIDS-----	
PASS #100 SIEVE---	8.0	DENSITY LBS PER CU FT	
PASS #200 SIEVE---	5.8	FLOW-----	
		STABILITY-----	

REMARKS: 3% CRUMB RUBBER

COPIES TO:	CHARGE NO.:	THIS REPORT INTENDED ONLY FOR
BIT. OFFICE	1125	INFORMATION AS TO UNIFORMITY OF
	1020	PRODUCTION MAKE NO CHANGES IN
	1111	RECOMMENDED ASPHALT PERCENTAGE
		WITHOUT CONTACTING BITUMINOUS ENGINEER.

SIGNED-----
ASST. CHIEF CHEMIST

STATE OF MINNESOTA
DEPARTMENT OF TRANSPORTATION

TEST REPORT ON SAMPLE OF BITUMINOUS CORE

LABORATORY----	ST. PAUL	REPORT DATE----	FEB 01, 1991
TESTS COMPLETED	01/26/91	PROJECT NUMBER-	9PR6002
SUBMITTED BY--	C TURGEON	TYPE OF CONST.-	2341
INSPECTOR-----		COURSE-----	
DATE SAMPLED--		STATION NO.----	
DATE RECEIVED-	01/07/91	FIELD ID-----	6,7,8

TESTS REQUIRED: EXTRACTION GRADATION SPECIAL TESTS
 COMMENTS: BIKEPATH 6,7,8

TEST RESULTS

SIEVE ANALYSIS(SQUARE OPENINGS)

% PASSING

PASS 1 IN. SIEVE--		BITUMEN(%)-----	5.3
PASS 3/4 IN. SIEVE		MOISTURE(%)-----	
PASS 5/8 IN. SIEVE	100.0	VOLATILE(%)-----	
PASS 1/2 IN. SIEVE	96.0	TESTS ON RECOVERED ASPHALT	
PASS 3/8 IN. SIEVE	83.0	PENETRATION 77 F.--	52
PASS #4 SIEVE-----	58.0	DUCTILITY 77 F.(CM)	
PASS #10 SIEVE----	47.0	SOFTENING POINT, F.	
PASS #20 SIEVE----	39.0	KVISC 275 F. CS----	
PASS #40 SIEVE----	28.0	AVISC 140 F. POISES	2125
PASS #80 SIEVE----	11.0	RICE VOIDS-----	
PASS #100 SIEVE---	9.0	DENSITY LBS PER CU FT	
PASS #200 SIEVE---	10.5	FLOW-----	
		STABILITY-----	

REMARKS:

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SIGNED-----
 ASST. CHIEF CHEMIST

STATE OF MINNESOTA
DEPARTMENT OF TRANSPORTATION

TEST REPORT ON SAMPLE OF BITUMINOUS CORE

LABORATORY-----	ST. PAUL	REPORT DATE-----	FEB 01, 1991
TESTS COMPLETED	01/29/91	PROJECT NUMBER-	9PR6002
SUBMITTED BY--	C TURGEON	TYPE OF CONST.-	2341
INSPECTOR-----		COURSE-----	
DATE SAMPLED--		STATION NO.-----	
DATE RECEIVED-	01/07/91	FIELD ID-----	10,11,12

TESTS REQUIRED: EXTRACTION GRADATION SPECIAL TESTS
COMMENTS: BIKEPATH 10,11,12

TEST RESULTS

SIEVE ANALYSIS(SQUARE OPENINGS)

% PASSING

PASS 1 IN. SIEVE--		BITUMEN(%)-----	7.8
PASS 3/4 IN. SIEVE		MOISTURE(%)-----	
PASS 5/8 IN. SIEVE	100.0	VOLATILE(%)-----	
PASS 1/2 IN. SIEVE	94.0	TESTS ON RECOVERED ASPHALT	
PASS 3/8 IN. SIEVE	81.0	PENETRATION 77 F.--	111
PASS #4 SIEVE-----	57.0	DUCTILITY 77 F.(CM)	
PASS #10 SIEVE-----	44.0	SOFTENING POINT, F.	
PASS #20 SIEVE-----	34.0	KVISC 275 F. CS----	
PASS #40 SIEVE-----	24.0	AVISC 140 F. POISES	268
PASS #80 SIEVE-----	9.0	RICE VOIDS-----	
PASS #100 SIEVE---	8.0	DENSITY LBS PER CU FT	
PASS #200 SIEVE---	5.7	FLOW-----	
		STABILITY-----	

REMARKS: 6% CRUMB RUBBER

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BIT. OFFICE	1125	
	1020	
	1111	

SIGNED-----
ASST. CHIEF CHEMIST

STATE OF MINNESOTA
DEPARTMENT OF TRANSPORTATION

TEST REPORT ON SAMPLE OF BITUMINOUS CORE

LABORATORY-----	ST. PAUL	REPORT DATE-----	FEB 01, 1991
TESTS COMPLETED	01/26/91	PROJECT NUMBER-	9PR6002
SUBMITTED BY--		TYPE OF CONST.-	2341
INSPECTOR-----		COURSE-----	
DATE SAMPLED--		STATION NO.-----	
DATE RECEIVED-	01/07/91	FIELD ID-----	C TURGEON

TESTS REQUIRED: EXTRACTION GRADATION SPECIAL TESTS
COMMENTS: BIKEPATH 14,15,16

TEST RESULTS

SIEVE ANALYSIS(SQUARE OPENINGS)

% PASSING

PASS 1 IN. SIEVE--		BITUMEN (%)-----	5.4
PASS 3/4 IN. SIEVE		MOISTURE (%)-----	
PASS 5/8 IN. SIEVE	100.0	VOLATILE (%)-----	
PASS 1/2 IN. SIEVE	96.0	TESTS ON RECOVERED ASPHALT	
PASS 3/8 IN. SIEVE	84.0	PENETRATION 77 F.--	34
PASS #4 SIEVE-----	56.0	DUCTILITY 77 F.(CM)	
PASS #10 SIEVE-----	46.0	SOFTENING POINT, F.	
PASS #20 SIEVE-----	38.0	KVISC 275 F. CS----	
PASS #40 SIEVE-----	27.0	AVISC 140 F. POISES	
PASS #80 SIEVE-----	13.0	RICE VOIDS-----	
PASS #100 SIEVE----	11.0	DENSITY LBS PER CU FT	
PASS #200 SIEVE----	7.8	FLOW-----	
		STABILITY-----	

REMARKS: 9% SHINGLES

COPIES TO:	CHARGE NO.:
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	1020
	1111

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SIGNED-----
ASST. CHIEF CHEMIST

STATE OF MINNESOTA
DEPARTMENT OF TRANSPORTATION

TEST REPORT ON SAMPLE OF BITUMINOUS CORE

LABORATORY----	ST. PAUL	REPORT DATE----	FEB 01, 1991
TESTS COMPLETED	01/26/91	PROJECT NUMBER-	9PR6002
SUBMITTED BY--	C TURGEON	TYPE OF CONST.-	2341
INSPECTOR-----		COURSE-----	
DATE SAMPLED--		STATION NO.----	
DATE RECEIVED-	01/07/91	FIELD ID-----	18,19,20

TESTS REQUIRED: EXTRACTION GRADATION SPECIAL TESTS
COMMENTS: BIKEPATH 18,19,20

TEST RESULTS

SIEVE ANALYSIS(SQUARE OPENINGS)

% PASSING

PASS 1 IN. SIEVE--		BITUMEN(%)-----	7.3
PASS 3/4 IN. SIEVE	100.0	MOISTURE(%)-----	
PASS 5/8 IN. SIEVE	99.0	VOLATILE(%)-----	
PASS 1/2 IN. SIEVE	96.0	TESTS ON RECOVERED ASPHALT	
PASS 3/8 IN. SIEVE	81.0	PENETRATION 77 F.--	55
PASS #4 SIEVE-----	52.0	DUCTILITY 77 F.(CM)	
PASS #10 SIEVE-----	41.0	SOFTENING POINT, F.	
PASS #20 SIEVE-----	33.0	KVIS 275 F. CS----	
PASS #40 SIEVE-----	23.0	AVIS 140 F. POISES	2548
PASS #80 SIEVE-----	9.0	RICE VOIDS-----	
PASS #100 SIEVE---	8.0	DENSITY LBS PER CU FT	
PASS #200 SIEVE---	5.5	FLOW-----	
		STABILITY-----	

REMARKS: 3% C RUBBER 6% SHING

COPIES TO:	CHARGE NO.:
BIT. OFFICE	1125
	1020
	111

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SIGNED-----
ASST. CHIEF CHEMIST

